

1 **EVALUATION OF NON-NUCLEAR SOIL MOISTURE AND DENSITY DEVICES FOR FIELD**
2 **QUALITY CONTROL**

3
4 **By:**

5
6 Ernest S. Berney IV, PhD, P.E.
7 US Army Engineer Research and Development Center
8 3909 Halls Ferry Rd
9 Vicksburg, MS 39180
10 (O): 601.634.3507
11 (F): 601.634.3020
12 Ernest.s.berney.IV@usace.army.mil
13

14 James D. Kyzar
15 WGK, Inc. Engineers and Surveyors
16 204 West Leake St.
17 Clinton, MS 39056
18 (O): 601.925.4444 ext. 240
19 (F): 601.924.6708
20 jkyzar@wgkengineers.com

21 **EVALUATION OF NON-NUCLEAR SOIL MOISTURE AND DENSITY DEVICES FOR FIELD**
22 **QUALITY CONTROL**

23

24 **Abstract**

25 When constructing new transportation infrastructure or maintaining current infrastructure systems,
26 achieving sufficient soil strength is critical to a successful construction effort. Currently, soil design specifications
27 are given in terms of a minimum soil density and a specified range of soil moisture content. Quality control is
28 achieved by monitoring the soil density and moisture content throughout the construction process. The Nuclear
29 Density Gauge (NDG) is the most commonly employed device to determine soil density and moisture content due to
30 its ease of use, speed of readings, and reliability of results. However, due to potential safety hazards and rigorous
31 user certification requirements, many agencies are seeking to replace the NDG. This paper focuses on a portion of a
32 much larger study that compares a wide range of compaction control devices, looking at the performance of devices
33 measuring only density and moisture content. Several new, commercially available alternatives for measuring soil
34 density were tested on a variety of soil types and conditions to determine the most consistent, well-performing
35 device. For the same soil types and conditions, several devices and techniques for determining soil moisture content
36 were also tested. The combination of the TransTech Soil Density Gauge and the heated fry-pan/open flame field
37 moisture content techniques represented the best alternative to the NDG.

38 **Background**

39 The compactive effort applied during soil construction has been established as the primary indicator of the
40 strength and performance of the constructed layer. The currently accepted “best” method of ensuring adequate soil
41 strength is through constant sampling of moisture content and dry density throughout the construction process. This
42 quality control (QC) activity is most commonly and expediently conducted using a nuclear density gauge (NDG)
43 (1), with accepted alternatives being the sand cone for density and a laboratory oven for moisture content
44 determination. Because of the regulatory and safety burdens required for using the NDG, various agencies tasked
45 with QC for horizontal construction including the Federal Highway Administration, and several state departments of
46 transportation (DOTs), have investigated available alternatives to the NDG. Recently, several large scale
47 investigations were performed, comparing volume replacement (2, 3) and electrical methods based on new
48 technologies including Time Domain Reflectometry (TDR) and Dielectrics (DI) (4-6). These devices are able to
49 provide a measured density and moisture content in the field without the regulatory burden imposed by use of the
50 nuclear gauge. Many other investigations have been performed to develop modulus-based devices for QC
51 applications (3, 7-10). Often these investigations are funded by an individual state DOT, with data collection
52 focused on the native soils of the sponsoring state. Many of these devices are able to provide reasonable
53 correlations for the soil of interest (3). Even after this research, there remains a demand for a device that can
54 measure soil density and moisture content comparable to the NDG over a broad range of soil types.

55 Development of soil moisture content devices has been episodic with new techniques developed followed
56 by a long lull in product development. One of the first techniques developed was the oven drying method (11). This
57 method is still viewed as the reference standard for determining moisture content of soils however the main
58 drawback to using the laboratory oven is the long time required to return results. Chemical methods were then
59 developed, such as the calcium carbide method, to provide expedient field moisture determination. The next
60 progression in measurement of moisture content were the developments of the NDG (1) followed by the microwave
61 oven moisture content test (12). While producing results faster than the standard oven, the microwave oven test still
62 requires removal of the soil from the field and a power supply for the microwave, capabilities often unavailable in
63 field applications. The most recent developments, TDR- and DI-based devices mentioned previously, can correlate
64 soil moisture content with the electrical properties of the soil (4,6).

66 **Methodology**

67 To provide a broad evaluation of the most successful commercially available equipment, eleven
68 compaction measurement devices and eight moisture-determining devices were identified. For compaction
69 determination, three were based on electrical methods, three on volume replacement methods, and five on
70 stiffness/impact methods. For moisture content determination, four were based on direct heat (gravimetric), three on
71 electronic, and one chemical. All compaction devices were referenced to the wet density obtained using the NDG,
72 that being the desired replacement device; moisture content, and subsequent dry density, were referenced to the
73 laboratory oven. Tables 1 and 2 list the selected density and moisture content devices, respectively, and their
74 associated categories. This exercise investigated their effectiveness on seven soil types to approximate typical soils
75 encountered during horizontal construction efforts for which the NDG serves an important role in QC. Table 3
76 presents a summary of the soils selected for testing and their associated engineering properties.

77 The research involved constructing a series of outdoor test sections to approximate real-world test
78 conditions, each 50-ft (15.2m) long by 12-ft (3.7m) wide, consisting of three 6-in.(15.2cm)-thick compacted lifts
79 such that the final test section was 18-in.(45.7cm)- thick (Figures 1, 2). The construction procedure provided a
80 suitable thickness of uniform soil above the natural subgrade to ensure that this layer did not adversely influence the
81 results of the various instruments tested. The first lift placed was approximately two roller widths across, about 12 ft
82 (3.7m), to provide a wide enough base to create a top layer at least 6-ft (1.8m) across. Eight coverages of a CAT
83 CS-443 roller were applied to the first, or bottom, lift and the second, or intermediate, lift. On the third, or top lift, a
84 single proof roll was performed across the entire width of the section tested as coverage number 1, with subsequent
85 coverages being applied as noted below.

86 For data collection, each soil test section was divided into four test regions, as shown in Figure 2. These
87 regions were contained within the central 40-ft (12.2m) of the test item (Figure 3). Each test region was 10-ft (3m)
88 long and 4-ft(1.2m) wide. Each device was used in each of the four regions to generate four replicate readings to
89 accommodate inherent variability in the soil and compaction process. Oven-dried moisture contents were taken at
90 each location where a density/modulus test was performed to allow normalization of response to changing
91 environmental conditions. Tests were conducted at coverages 1, 2, 4, and 8 to provide varying density conditions in
92 each of the four regions. Following these tests, the test sections were soaked with a portable water sprayer and
93 retested in a more moist condition for different field moisture content (Figure 2).

94

95 Calibration and Use of Devices

96

97 Density/Modulus Devices and Techniques

98

99 Electrical Moisture-Density Devices

100 The EDG and M+DI measure electrical resistance between a series of probes embedded in a soil (4). The resultant
101 resistance is compared to a set of calibrated readings covering the range of field moisture contents and wet densities
102 expected in the field. The SDG is a plate that rests above the soil surface and computes the impedance of the soil
103 based on statistical factors from the frequency sweep generated during testing (6). To function in the field, each
104 device must be calibrated to some known soil physical data. The EDG requires internal correlations to be developed
105 between the expected field range of moisture-density conditions measured with the NDG (or some other density
106 standard) and the resultant soil resistance. For this study, the correlation was achieved by comparing three data
107 points in the field: at the first and the last pass of the second lift (low and high density) and on loose soil dried to the
108 side of the test section (low moisture). The M+DI was calibrated using built-in data sets that approximated the
109 moisture-density response in the field. A proper ASTM laboratory calibration was planned; however during field
110 testing, the M+DI often provided null readings, especially in granular soils. Due to this, the M+DI was not further
111 calibrated or evaluated for moisture content and since this study was undertaken, has been removed from the
112 marketplace. The SDG was calibrated using soil index properties based on laboratory testing to provide an initial
113 reading of the device. A corrected SDG reading (SDG-Corr) was later obtained by including a linear offset factor
114 derived from the dry density and moisture content readings obtained from the first sand cone test and the laboratory
115 oven test conducted on each soil type. All of these electrical devices provided the user the wet density, dry density,
116 and moisture content of the soil, resulting in a comparative data set to the NDG.

117

118 Volume Replacement Devices

119 Volume replacement techniques for density determination are useful due to the lack of calibration required for use.
120 However, these techniques are especially sensitive to the hole dug to determine the volume of removed soil. The
121 sand cone (SC) and water balloon (WB) have been identified as the most common volume replacement devices,
122 with the steel shot replacement (SS) being a recent military development (13). Each technique measures a weight of
123 wet soil excavated from a hole and uses a known volume of material to fill the hole. Dividing the wet soil weight by
124 the excavated volume provides a wet density for the soil. A sample of the wet soil is dried in an oven to obtain the
125 moisture content. Finally, the wet density and water content are used to obtain the dry density of the soil, resulting
126 in a comparative data set to the NDG.

127

128 Stiffness/Modulus Devices

129 Each of the instruments in this category returns varying responses. The DCP provides a number of hammer drops
130 per depth of penetration (14,15). For the analysis, a summary total of hammer drops over a nominal 6-in. depth of
131 penetration is used as the comparative value to the density. The CH returns a Clegg Index value (CIV) that
132 corresponds to the acceleration response at the point of impact of the hammer (8). This is the value used as the
133 comparison to density. The Zorn and Dynatest LWD devices return a deflection measurement of the plate and a
134 back-calculated soil modulus based on an assumed Poisson's ratio. However, only the deflection was used as a
135 comparative measure to density in this research, as the back-calculated modulus is a derived data output from the
136 devices. The GG returns a modulus (force/area) and a stiffness (force/length), only one of which was chosen as the
137 representative output for the device (16). After considerable analytical effort during this project, it was confirmed
138 that no clear correlations existed between modulus/stiffness values of soil and the Proctor density-moisture
139 relationship. This is an issue that is well documented in the literature (3,17) and is an area for continued research.
140 Therefore, only the electrical and volume replacement devices are offered for side-by-side comparison to the NDG.
141 Details of the modulus/stiffness observations and response data can be found in the full technical report (18).

142

143 Moisture Content Devices and Techniques

144

145 Electrical Devices

146 The EDG and the SDG, previously described, are the two electrical devices fully evaluated in this study.

147

148 Gravimetric Devices

149 These devices consist of drying technologies including convection (laboratory oven), radiation (standard and field
 150 microwaves), and conduction from a ceramic heating element (moisture analyzer) or a gas flame burner (gas stove).
 151 The microwaves and gas burner require a series of manual measurements to be made during the drying process to
 152 determine the final constant dry mass of the soil. This technique is assisted by the use of software developed
 153 specifically for these types of drying used in the U.S. Army Engineer Research and Development Center's Rapid
 154 Soils Analysis Kit (19). The software prompts the user for mass of the soil at one minute drying increments and
 155 internally calculates the mass differential between drying times until a prescribed minimum difference is obtained.
 156 For field use, the threshold is considered as less than 1% of the total wet mass. For the associated microwave
 157 ASTM specification, the threshold is nearer to 0.1% of the total wet mass. The 1% value is used for military
 158 consideration based on an outdoor scenario in which wind and environmental conditions prevent measurement
 159 accuracy consistent enough to measure a 0.1% differential, especially with small soil specimens.

161 **Chemical Devices**

162 The Speedy moisture tester indirectly measures the moisture content of soil by determining the amount of acetylene
 163 gas produced by a reactant material and the free moisture in the soil. The Speedy moisture uses calcium carbide as
 164 the reactant material. The device measures the amount of acetylene gas produced by recording the pressure change
 165 in a steel vessel during the water-chemical reaction. This pressure change is then related to the mass of water and
 166 ultimately the gravimetric moisture content, assuming all free water in the soil has reacted with the calcium carbide.

168 **Collection of Data**

169 For all density devices tested, a soil sample was taken from the top 2 to 4 in. (5 to 10cm) of the ground
 170 surface at the point of each device measurement to normalize any data response with laboratory oven moisture
 171 content. For the electrical devices, the field moisture content allowed a comparison to be made directly to the
 172 laboratory oven. For the NDG, the wet density is assumed to be the most accurate measurement, and so the oven
 173 moisture content represented a check against this device's measured moisture and was used to calculate the dry
 174 density at each test location. The moisture content from the laboratory oven was used to calculate the dry density of
 175 all the volume replacement techniques, as this was a required part of their testing procedure.

176 To conduct the moisture content study, a bulk sample was taken from the stockpile of each soil used during
 177 construction. The samples were placed in one-gallon metal paint cans and sealed until moisture measurements were
 178 conducted. Each paint can held about 4-5 kg (9-11 lbs) of soil, which was enough to provide at least three replicate
 179 experiments using 200-250 g (7-9oz) of material for each of six test devices. The paint cans remained sealed for 4
 180 to 6 weeks, during which time the moisture had an opportunity to equilibrate throughout the bulk soil specimen.
 181 When testing began on a particular soil, three random samples of soil were extracted from the can for determination
 182 of moisture content by the laboratory oven method. The average moisture content of these samples was considered
 183 to be the reference moisture content for the bulk sample within the can. All of the remaining samples were treated
 184 similarly, with three random samples of soil drawn from the can and tested. The NDG, EDG, and SDG were all
 185 tested based on their responses during the large-scale density study. All collected data and analyses can be viewed
 186 in greater detail in a complementary technical report (20).

188 **Experimental Results and Analysis**

190 *Density Devices*

191
 192 Though the overall testing lasted over a period of two months, all seven test sections were constructed and
 193 tested concurrently with construction. For each soil type and density/modulus device, 20 data points were collected:
 194 four replicates at each of four coverage levels and the final soaked condition. The evaluation of the compaction
 195 monitoring devices involved comparing the dry densities obtained by each device to the dry density obtained by the
 196 NDG as this was the device sought for replacement. In all cases, the dry density was determined by converting all
 197 reported wet densities, γ_w , to dry density, γ_d , as shown in Equation 1 using the laboratory oven moisture content
 198 (MC_{lab}) as the reference standard.

$$200 \quad \gamma_D = \gamma_W \div (1 + MC_{lab}) \quad (1)$$

201
 202 The initial analysis identified the maximum percentage of outliers for each device to define performance based
 203 on reliability. It was found that the WB and M+DI both exhibited greater than 25% null readings (or outliers) and

204 therefore were considered inadequate replacements for the NDG and removed from further consideration. The next
 205 step in the comparison process was to determine the percent device density deviation from the NDG for the
 206 remaining density devices, SC, SS, EDG, SDG, and SDG-Corr. The percent deviation was used because it would
 207 return a dimensionless performance indicator. Due to the variable nature of QC in soils and testing over increasing
 208 levels of compaction, spreads in deviation readings are more indicative of the ability of a device to present reliable
 209 data than are the absolute readings themselves. The spreads in deviation were calculated as shown in the flowchart
 210 in Figure 4. This approach was taken because each device was tested randomly within a 40-ft² test region, but not at
 211 the same location as the NDG. Therefore a one-to-one comparison of device to NDG is not representative of device
 212 response as there is variability of density and moisture within the test section leading to variability between NDG
 213 readings for a single coverage level and device variability.

214 The spreads for the density devices for a particular soil (average and max-min) were ranked in increasing order
 215 from 1 to 5 with 1 being the best and 5 the worst. The rankings for each device for all soil types were then added to
 216 yield a composite rank for comparative analysis as shown in Figures 5 and 6 for the average high-low and max-min
 217 values, respectively. The figures show the individual rankings for each soil type, with the lowest scoring devices
 218 performing significantly better than the highest scoring devices.

219

220 *Moisture Devices*

221

222 Processing of data from the moisture content experiment occurred in two phases. The first indicator of a
 223 device's performance was its ability to capture the moisture content value compared to the laboratory oven method
 224 (accuracy). To determine this metric, the slope of the device's moisture content plotted against the laboratory oven's
 225 moisture content (unity) was determined for all soil types. The Bias of a device's performance (Equation 2) was
 226 based on the absolute slope differential between the device's measured slope and unity. This comparison can be
 227 seen in Figure 7. Slopes approaching m=1 indicate overall agreement with the values from the laboratory oven,
 228 whereas m<1 indicates under-prediction and m>1 indicates over-prediction of moisture content. Table 4 shows
 229 device data as compared to that of the laboratory oven including the slope, slope offset from laboratory oven, and
 230 the standard deviation of the device/lab oven ratio. The second indicator of a device's performance was the
 231 deviation of measured values from the average moisture content (precision). To determine this metric, the ratio of
 232 device moisture content to the average lab oven moisture content was taken. The standard deviation for these ratios
 233 was then found for each soil as shown in Figure 8. Soils are ranked in order of increasing average grain size.

234 To combine the accuracy and precision of each device for moisture content, the metric of Total Analytical
 235 Error (TAE) was employed. The calculation for TAE is shown in Equations 2 and 3. Figure 9 shows the final
 236 metric for each device. Devices with a lower TAE have a better combination of accuracy and precision than devices
 237 with a higher TAE.

238

$$239 \quad Bias = \left| \frac{1 - slope}{1} \right| \quad (2)$$

240

$$241 \quad TAE = Bias + \frac{\sigma}{\bar{X}} \quad (3)$$

242

243 Where:

244 Bias= Absolute value of the slope offset from the desired slope, normalized to the desired slope.

245 TAE= Combination of the accuracy and precision of the measurements

246 σ =Overall standard deviation of the device to lab oven ratio247 \bar{X} =Average of all devices to lab oven ratios

248

249 **Conclusions**

250

251 *Moisture Content*

252 When calibration against the laboratory oven is possible, the SDG and NDG both provide devices that can
 253 return accurate and reliable density and moisture content values. When calibration is not available, the gas stove or
 254 microwave ovens represent the best field devices. These devices can also be considered as alternatives for use in
 255 calibration of the recommended electronic gauges. The moisture analyzer and Speedy are not considered reliable
 256 field devices over the full spectrum of soils encountered in construction. Devices that did not perform well usually

257 failed when the physical structure of the soils allowed for greater variation in moisture content, such as with the
 258 coarse-grained soils. Fine-grained soils and non-plastic soils, such as the silts, tended to yield accurate
 259 measurements by more devices. The uniform heating of the gas stove, as opposed to the interrupted microwave
 260 heating, created a much more reliable set of moisture measurements.

261 262 *Density*

263 Considering that if a device is to replace the NDG, it should have performance approaching or exceeding
 264 that of the NDG and therefore all devices were compared to the performance of the NDG. Based on Figures 5 and
 265 6, the corrected SDG-Corr proved to have the least variability in both the average value for each soil and the least
 266 amount of high-low scatter from the average value and was deemed the best electronic substitute for the NDG. The
 267 sand cone was the next best device overall and deemed the best volumetric replacement device compared to the
 268 NDG. The EDG performed well but required a more complex calibration routine to establish its accuracy. The steel
 269 shot test proved to have the greatest variability in the soils tested, although this device is intended primarily for
 270 contingency measurements allowing for a larger amount of scatter in return for a more expedient test procedure. It
 271 should be noted that the uncorrected SDG experienced considerably more variability than the EDG or sand cone,
 272 indicating a lack of sufficient internal calibration for the soils tested.

273 Overall, the ability to capture both moisture and density with a single device rather than with both a heating
 274 device and a field density apparatus increases the value of the electronic devices as a single solution for replacing
 275 the NDG for construction QC.

276 277 **References**

- 278 1. Belcher, D.J. and T.R. Cuykendall. 1950. *The measurements of soil moisture and density by neutron and*
 279 *gamma-ray scattering*. Civil Aero. Adm. Tech. Dev. Rept. 127, Washington D.C.
- 280 2. Sebesta, S., C. Estakhri, T. Scullion, and W. Liu. 2006. *New Technologies for Evaluating Flexible Pavement*
 281 *Construction: Year 1 Report*. FHWA/TX-06/0-4774-1. College Station, TX: Texas Department of
 282 Transportation.
- 283 3. Rathje, E.M., S.G. Wright, K. H. Stokoe II, A. Adams, R. Tobin, and M. Salem. 2006. *Evaluation of non-*
 284 *nuclear methods for compaction control*, FHWA/TX-06/0-4835-1. College Station, TX: Texas Department of
 285 Transportation.
- 286 4. Lin, CP, V. P. Drnevich, W. Fen, and R. J. Deschamps. 2000. Time Domain Reflectometry for Compaction
 287 Quality Control. *ASCE Proceedings, Geo-Denver 2000*.
- 288 5. Brown, J. 2007. Non-nuclear compaction gauge comparison study. *VTrans 2007-19*. Montpelier, VT:
 289 Vermont Agency of Transportation.
- 290 6. Gamache, R. W., E. Kianirad, S. Pluta, S.R. Jersey, and A.N. Alshawabkeh. 2009. A rapid field soil
 291 characterization system for construction control. *Transportation Research Record: Journal of the*
 292 *Transportation Research Board*, (617), 1-12.
- 293 7. Crovetto, J.A. 2002. Deflection-based analysis techniques for jointed concrete pavement systems.
 294 *Transportation Research Record No. 1809*, Washington, D.C.: Transportation Research Board, National
 295 Research Council, , pp 3-11.
- 296 8. Mooney, M.A., C.S. Nocks, K.L. Selden, G.T. Bee, and C.T. Senseney 2008. *Improving quality assurance of*
 297 *MSE wall and bridge approach earthwork compaction*. CDOT-2008-11. Denver, CO: Colorado Department
 298 of Transportation.
- 299 9. Tehrani, F.S., and C. L. Meehan. 2010. The effect of water content on light weight deflectometer
 300 measurements. *GeoFlorida 2010: Advances in Analysis, Modeling, and Design*. West Palm Beach, FL.
- 301 10. Siekmeier, J., C. Pinta, S. Meth, J. Jensen, P. Davich, F. Camargo, and M. Beyer. 2009. Using the Dynamic
 302 Cone Penetrometer and the Light Weight Deflectometer for Construction Quality Assurance. *Minnesota Dept.*
 303 *of Transportation MN/RC 2009-12*, Final Report, Feb. 2009.
- 304 11. Buchanan, S. J. 1939. Technique of Soil Testing. *Civil Engineer*, 9, 568-572.
- 305 12. Miller, R. J., R. B. Smith, and J. W. Biggar. 1974. Soil water content: microwave oven method. *Soil Science*
 306 *Society of America*, 38, 535-537.
- 307 13. Freeman, R. B., C. A. Gartrell, L. D. Wakeley, E.S. Berney IV, and J. R. Kelley. (2010). Steel-shot method for
 308 measuring the density of soils. *Canadian Geotechnical Journal*, 47(11), 1299-1304.
- 309 14. Webster, S.L., R.H. Grau, and T.P. Williams. 1992. *Description and application of a dual mass dynamic cone*
 310 *penetrometer*. Instruction report GL-92-3. Vicksburg, MS: U.S. Army Engineer Waterways Experiment
 311 Station

- 312 15. Jayawickrama, P., A. Amarasiri, and P. Regino. 2000. Use of dynamic cone penetrometer to control
313 compaction of granular fill. *Transportation Research Record: Journal of the Transportation Research Board*,
314 1736(1), 71-80.
- 315 16. Lenke, L. R., R.G. McKeen, and M. P. Grush. 2003. Laboratory evaluation of GeoGauge for compaction
316 control. *Transportation Research Record: Journal of the Transportation Research Board*, 1849, 20-30.
- 317 17. Khoury, N., and M. Zaman. 2004. Correlation between resilient modulus, moisture variation, and soil suction
318 for subgrade soils. *Transportation Research Record: Journal of the Transportation Research Board*, 1874(1),
319 99-107.
- 320 18. Berney IV, E.S., J.D. Kyzar, M. Mejias, C. Roig-Silva, A. Manning, E. Villanueva, J. Rowland, C. Bradley and
321 S.R. Jersey. 2011. *Non-nuclear alternatives to monitoring moisture-density response in soils*. ERDC/GSL TR-
322 12-XX, Vicksburg, MS: U.S. Army Engineer Research and Development Center, Waterways Experiment
323 Station.
- 324 19. Berney IV, E. S. and R.E. Wahl. 2008. *A rapid soils analysis kit*. ERDC/GSL TR-08-3, Vicksburg, MS: U.S.
325 Army Engineer Research and Development Center, Waterways Experiment Station.
- 326 20. Berney IV, E. S., J.D. Kyzar and L.O. Oyelami. 2011. *Device comparison for determining field soil moisture*
327 *content*, ERDC/GSL TR-11-XX, Vicksburg, MS: U.S. Army Engineer Research and Development Center,
328 Waterways Experiment Station.

329

330 Acknowledgement

331 The tests described and the resulting data presented herein, unless otherwise noted, were obtained from research
332 conducted for the Air Force and performed at the U.S. Army Engineer Research and Development Center.
333 Permission was granted by the Director of the GSL to publish this information.

List of Tables and Figures**TABLE 1 Density/Modulus Devices and Techniques****TABLE 2 Moisture Content Devices and Techniques****TABLE 3 Soil Types Tested****TABLE 4 Device Moisture Content vs Laboratory Oven****FIGURE 1 Test layout for all soils.****FIGURE 2 Test Execution.****FIGURE 3 Individual soil showing four testing regions.****FIGURE 4 Flowchart indicating approach to measuring device performance.****FIGURE 5 Average Dry Density Spread Value Overall Ranking.****FIGURE 6 Maximum-Minimum Dry Density Spread Value Overall Ranking.****FIGURE 7 Ratio of average device to laboratory oven moisture content for each soil tested.****FIGURE 8 Standard deviation of moisture content for each tested device for each soil type tested.****FIGURE 9 Rating statistic for moisture content as the product of the slope offset and standard deviation for all tested devices.**

TABLE 1 Density/Modulus Devices and Techniques

Wet Density & Moisture Content	Wet Density only	Modulus or Stiffness
Moisture Density Indicator (MDI) Electrical Density Gauge (EDG) Soil Density Gauge (SDG) Nuclear Density Gauge (NDG)	Water Balloon (WB) Sand Cone (SC) Steel Shot (SS)	Clegg Hammer (CH) GeoGauge (GG) Dynatest Lightweight Deflectometer (D-LWD) Zorn Lightweight Deflectometer (Z-LWD) Dynamic Cone Penetrometer (DCP)

TABLE 2 Moisture Content Devices and Techniques

Electronic	Direct Heat (Gravimetric)	Chemical
Electrical Density Gauge (EDG) Soil Density Gauge (SDG) Nuclear Density Gauge (NDG)	Laboratory Oven Lab Microwave Field Microwave Gas Stove Moisture Analyzer	Speedy Moisture

TABLE 3 Soil Types Tested

Descriptor	USCS Class.	Grain Size Percentage by Weight				Atterberg Limits		Standard Proctor	
		Gravel	Sand	Silt	Clay	LL	PL	OMC (%)	MDD (pcf)/(gm/cm ³)
Crushed Limestone	GP-GM	52.8	40.9	3.9	2.4	15	12	6.8	136.3/(2.183)
Silty Gravel	SM	29.2	45.9	21.1	3.8	NP	NP	7.8	129.7/(2.076)
Clay Gravel	SP-SC	41.3	50.7	3.1	4.9	23	13	8	128.8/(2.063)
Silty Sand	ML-2	2.7	47	43.9	6.4	NP	NP	10	121.8/(1.951)
Concrete Sand	SP	4.9	36.1	2.3	0.8	NP	NP	9.5	109/(1.746)
Vicksburg Loess	ML-1	1.2	11	78.4	9.4	NP	NP	15.8	109.5/(1.754)
Buckshot Clay	CH	0	4.9	18.6	76.5	73	24	24.6	85.7/(1.373)

TABLE 4 Device Moisture Content vs Laboratory Oven

Device	Slope	Slope Offset (Slope-1)	Standard Deviation of Device to Laboratory Oven Ratio
Lab Oven	1.000	0.000	0.089
NDG	0.922	-0.078	0.108
Gas Stove	1.027	0.027	0.213
SDG (corr)	0.979	-0.021	0.253
Field Microwave	0.897	-0.103	0.170
STD Microwave	1.091	0.091	0.222
EDG	1.010	0.010	0.318
Moisture Analyzer	0.731	-0.269	0.238
Speedy	1.405	0.405	0.260

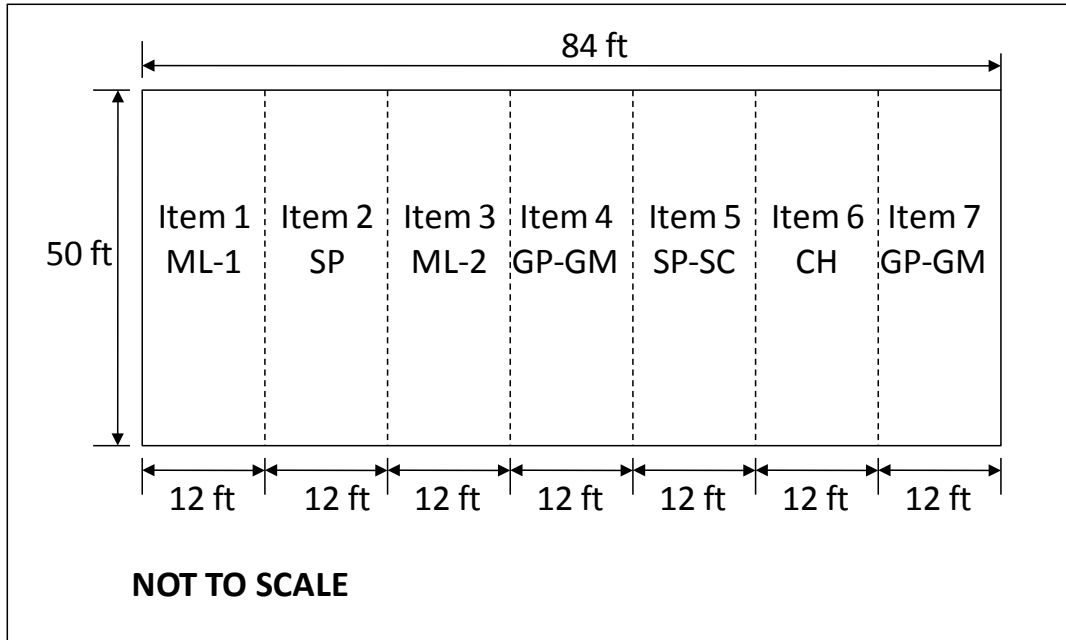


FIGURE 1 Test layout for all soils.



FIGURE 2 Test Execution. Top left: Device testing on ML-1. Top right: Construction of SP-SC layer (final lift). Bottom left: Construction of GP-GM with test regions marked. Bottom right: Soaking of ML-1 test section following testing of 8th coverage.

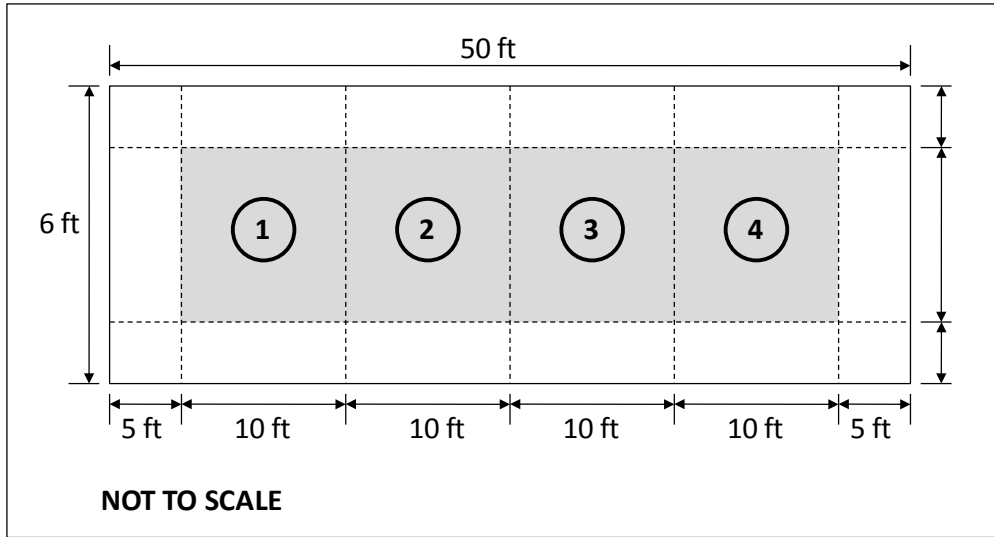


FIGURE 3 Individual soil showing four testing regions.

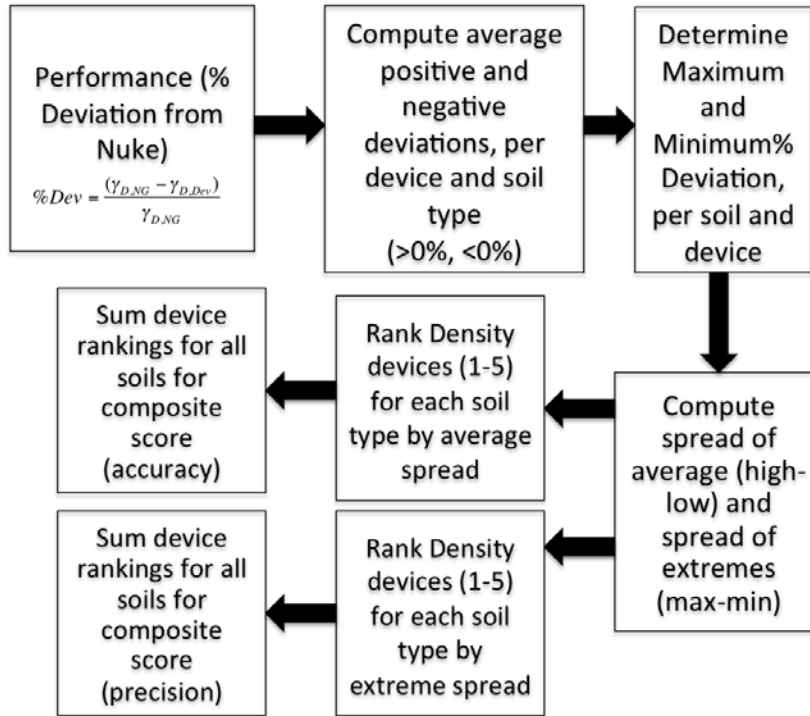


FIGURE 4 Flow chart indicating approach to measuring device performance.

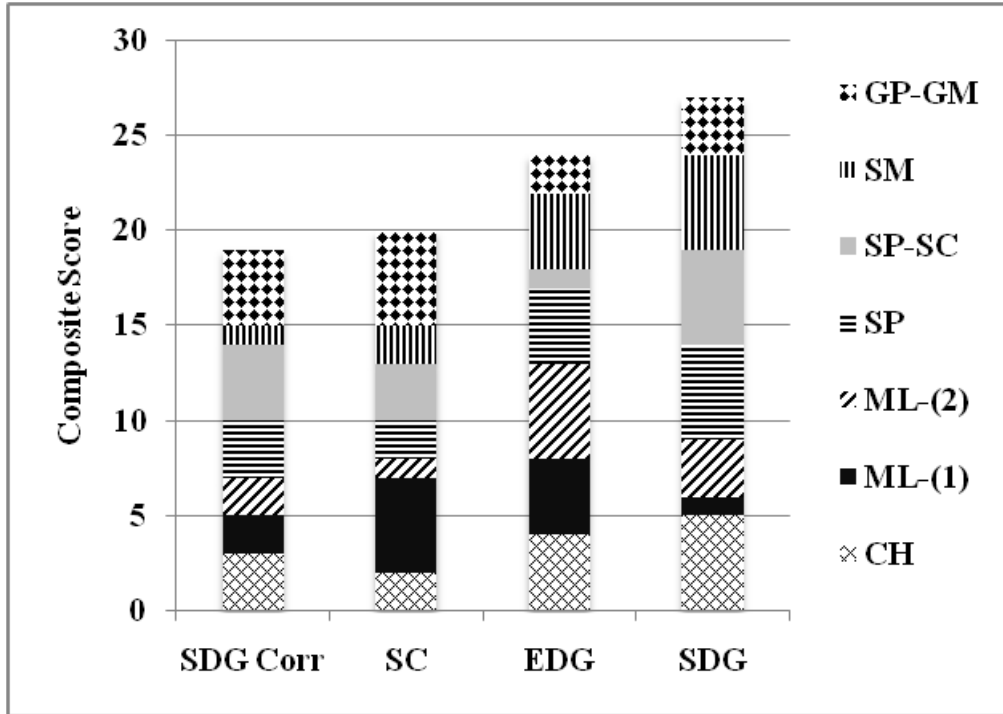


FIGURE 5 Average Dry Density Spread Value Overall Ranking.

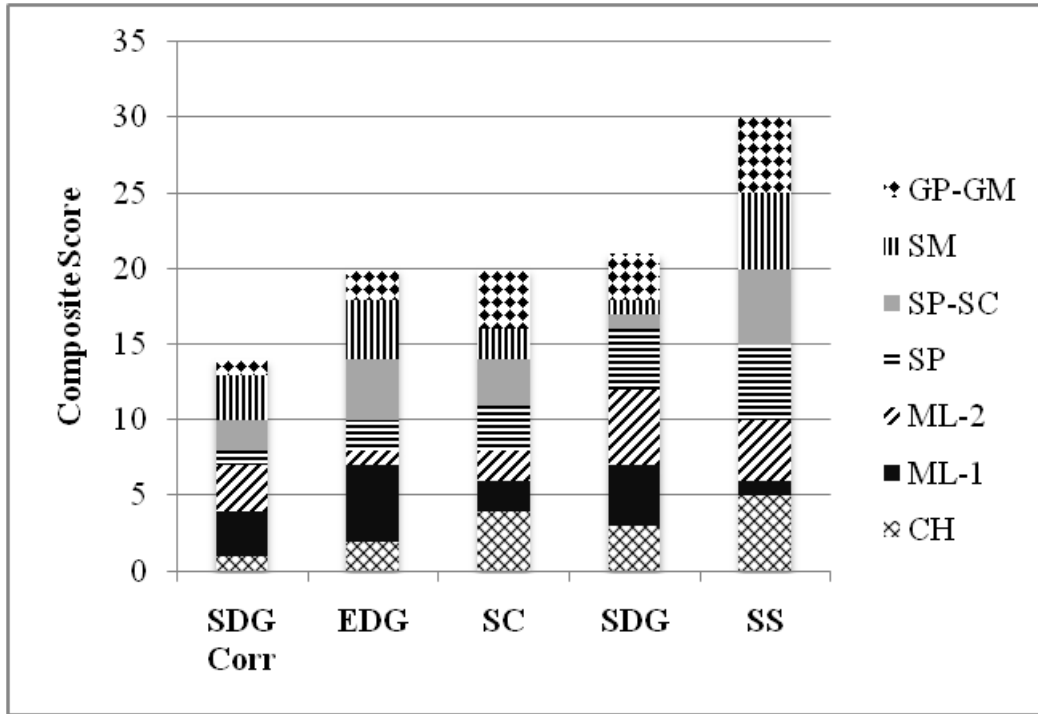


FIGURE 6 Maximum-Minimum Dry Density Spread Value Overall Ranking.

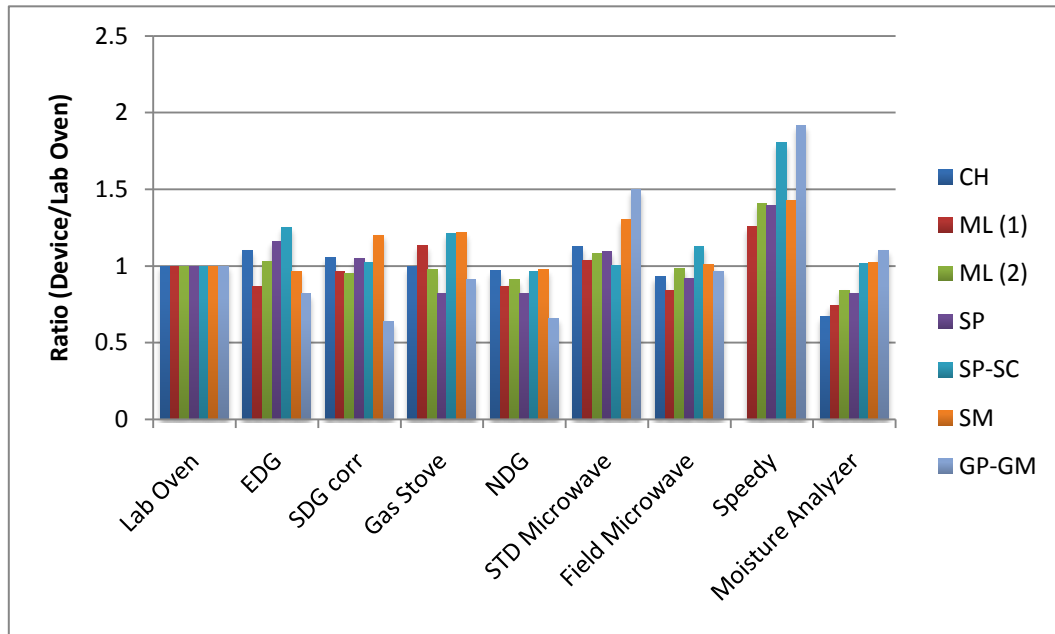


FIGURE 7 Ratio of average device to laboratory oven moisture content for each soil tested.

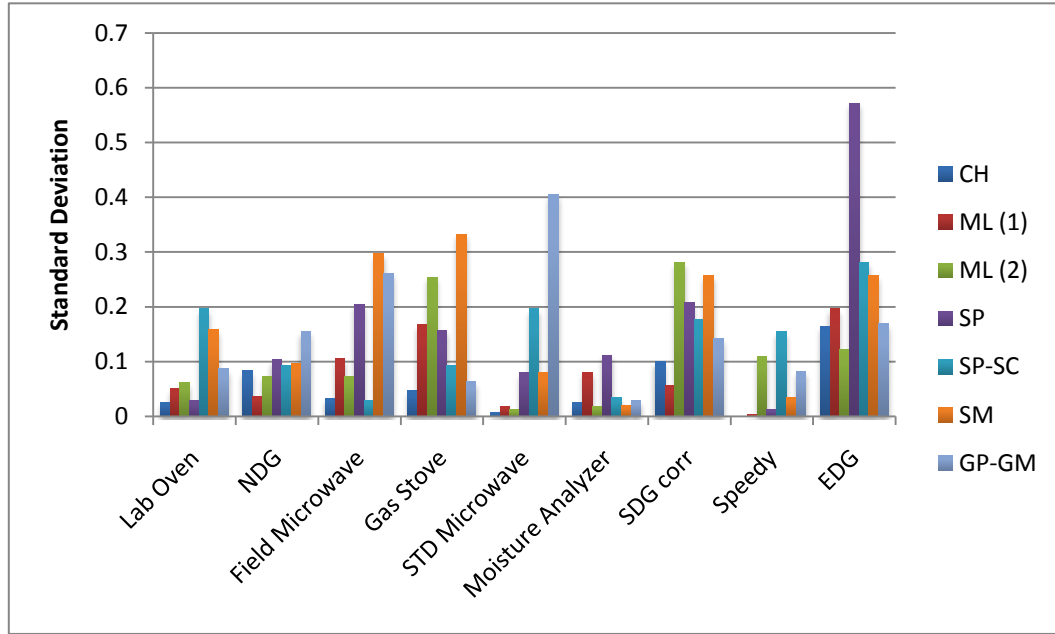


FIGURE 8 Standard deviation of moisture content for each tested device for each soil type tested.

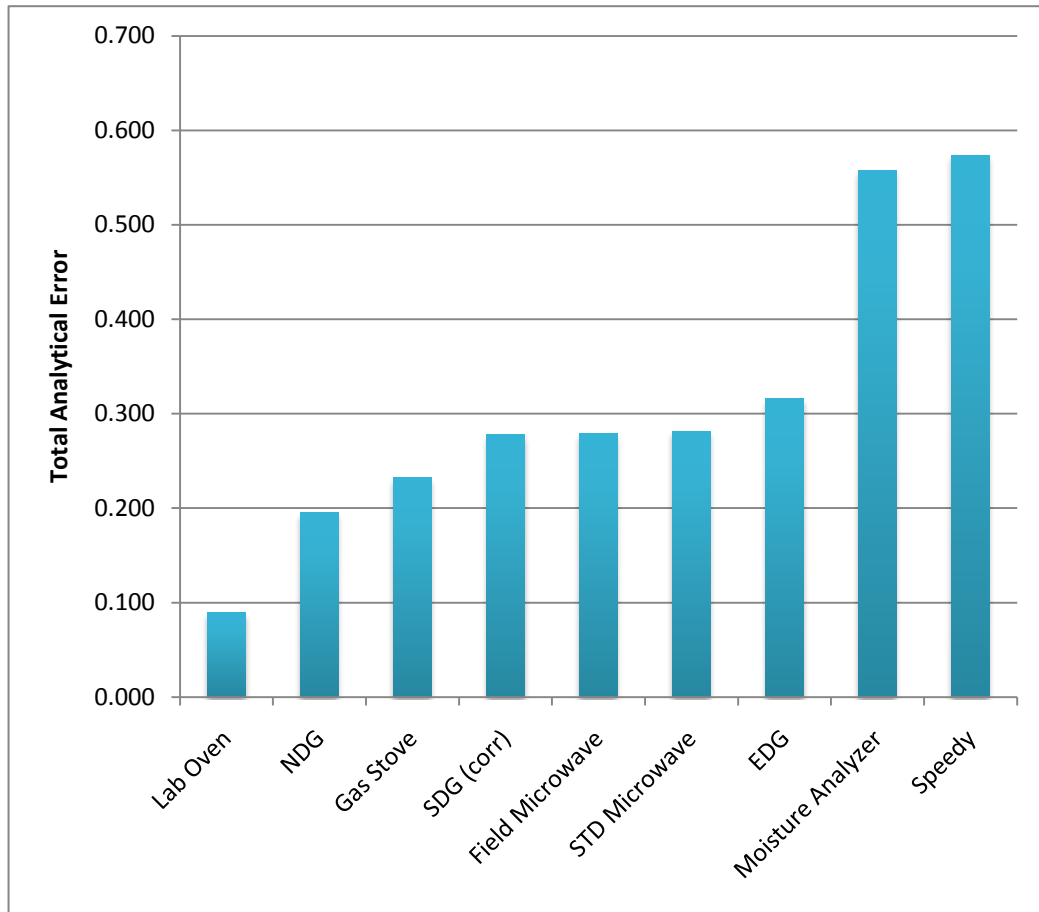


FIGURE 7 Rating statistic for moisture content as the product of the slope offset and standard deviation for all tested devices.